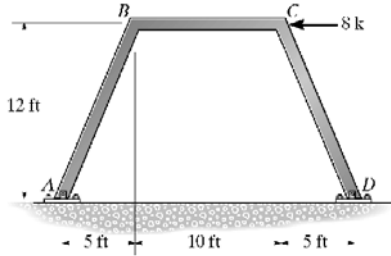


## Slope-deflection examples with sloping members.

Note that in the first example, the pinned support conditions for the columns were taken advantage of and the “short-hand” slope-deflection formula is used in calculating the  $M_{BA}$  and  $M_{CD}$ .

### Example 1

Determine the moment at each joint of the batter-column frame. The supports at A and D are pins.  $EI$  is constant.



$$(FEM)_{BA} = (FEM)_{BC} = (FEM)_{CB} = (FEM)_{CD} = 0$$

$$\psi_{AB} = \psi_{DC} = \frac{\Delta}{13} \quad \psi_{BC} = \frac{2\Delta \cos 67.38^\circ}{10}$$

$$\psi_{AB} = \psi_{DC} = \psi_{BC}$$

$$M_N = 3E\left(\frac{I}{L}\right)(\theta_N - \psi) + (FEM)_N$$

$$M_{BA} = 3E\left(\frac{I}{13}\right)(\theta_B + \psi_{AB}) + 0$$

$$M_{BA} = 0.2308EI(\theta_B + \psi_{AB}) \quad (1)$$

$$M_N = 2E\left(\frac{I}{L}\right)(2\theta_N + \theta_F - 3\psi) + (FEM)_N$$

$$M_{BC} = 2E\left(\frac{I}{10}\right)(2\theta_B + \theta_C - 3\psi_{AB}) + 0$$

$$M_{BC} = 0.2EI(2\theta_B + \theta_C - 3\psi_{AB}) + 0 \quad (2)$$

$$M_{CB} = 2E\left(\frac{I}{10}\right)(2\theta_C + \theta_B - 3\psi_{AB}) + 0$$

$$M_{CB} = 0.2EI(2\theta_C + \theta_B - 3\psi_{AB}) + 0 \quad (3)$$

$$M_N = 3E\left(\frac{I}{L}\right)(\theta_N - \psi) + (FEM)_N$$

$$M_{CD} = 3E\left(\frac{I}{13}\right)(\theta_C + \psi_{AB}) + 0$$

$$M_{CD} = 0.2308EI(\theta_C + \psi_{AB}) \quad (4)$$

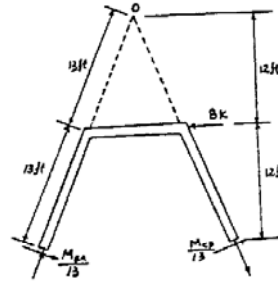
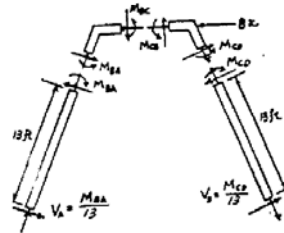
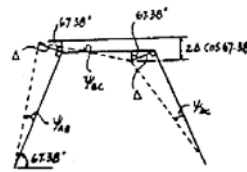
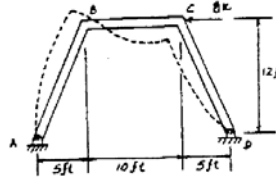
Equilibrium

$$M_{BA} + M_{BC} = 0 \quad (5)$$

$$M_{CD} + M_{CB} = 0 \quad (6)$$

$$\sum \mathcal{M}_O = 0: \frac{M_{BA}}{13}(26) + \frac{M_{CD}}{13}(26) - 8(12) = 0$$

$$2M_{BA} + 2M_{CD} - 96 = 0 \quad (7)$$



Solving these equations :

$$\theta_B = \theta_C = \frac{32}{EI}$$

$$\psi_{AB} = \frac{72}{EI}$$

$$M_{BA} = 24 \text{ k}\cdot\text{ft} \quad \text{Ans}$$

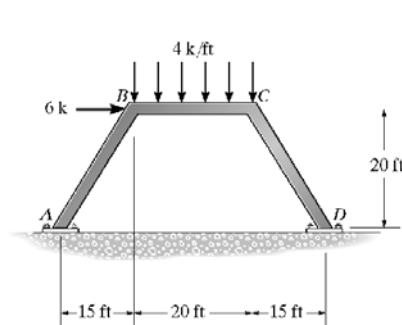
$$M_{BC} = -24 \text{ k}\cdot\text{ft} \quad \text{Ans}$$

$$M_{CB} = -24 \text{ k}\cdot\text{ft} \quad \text{Ans}$$

$$M_{CD} = 24 \text{ k}\cdot\text{ft} \quad \text{Ans}$$

## Example 2

For the battered-column frame, determine the moments at each joint and at the fixed supports A and D.  $EI$  is constant.



$$\psi_{AB} = \frac{\Delta}{25} \quad \psi_{DC} = \frac{\Delta}{25}$$

$$\psi_{BC} = \frac{2\Delta \cos 53.13^\circ}{20} = 0.06\Delta$$

$$\psi_{AB} - \psi_{DC} = 0.6667\psi_{BC} \quad (1)$$

$$(FEM)_{AB} = (FEM)_{BA} = 0$$

$$(FEM)_{BC} = \frac{-4(20)^2}{12} = -133.33 \text{ k} \cdot \text{ft}$$

$$(FEM)_{CB} = \frac{4(20)^2}{12} = 133.33 \text{ k} \cdot \text{ft}$$

$$(FEM)_{CD} = (FEM)_{DC} = 0$$

$$M_N = 2E\left(\frac{I}{L}\right)(2\theta_N + \theta_F - 3\psi) + (FEM)_N$$

$$M_{AB} = \frac{2EI}{25}(0 + \theta_B - 3\psi_{AB}) + 0 \quad (2)$$

$$M_{BA} = \frac{2EI}{25}(2\theta_B + 0 - 3\psi_{AB}) + 0 \quad (3)$$

$$M_{BC} = \frac{2EI}{20}(2\theta_B + \theta_C + 3\psi_{BC}) - 133.3 \quad (4)$$

$$M_{CB} = \frac{2EI}{20}(2\theta_C + \theta_B + 3\psi_{BC}) + 133.3 \quad (5)$$

$$M_{CD} = \frac{2EI}{25}(2\theta_C + 0 - 3\psi_{DC}) + 0 \quad (6)$$

$$M_{DC} = \frac{2EI}{25}(0 + \theta_C - 3\psi_{DC}) + 0 \quad (7)$$

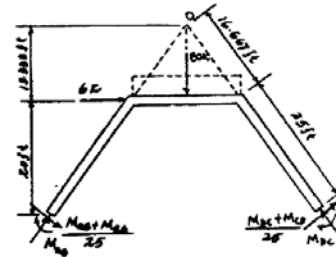
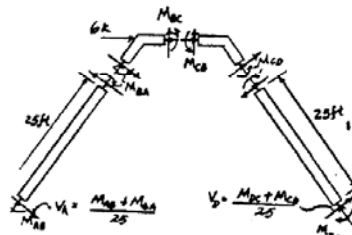
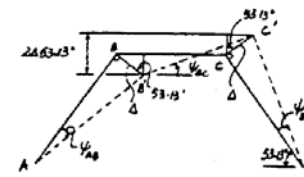
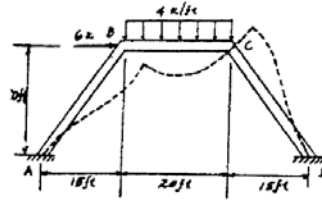
Equilibrium :

$$M_{BA} + M_{BC} = 0 \quad (8)$$

$$M_{CB} + M_{CD} = 0 \quad (9)$$

$$\sum M_O = 0: \left(\frac{M_{AB} + M_{BA}}{25}\right)(41.667) + \left(\frac{M_{DC} + M_{CD}}{25}\right)(41.667) - M_{AB} - M_{DC} + 6(13.333) = 0$$

$$0.6667M_{AB} + 1.6667M_{BA} + 1.6667M_{CD} + 0.6667M_{DC} = -80 \quad (10)$$



Solving Eqs. 1 - 10,

$$\theta_B = \frac{487.0}{EI}$$

$$\theta_C = \frac{-538.7}{EI}$$

$$\psi_{AB} = \psi_{CD} = \frac{56.65}{EI}$$

$$\psi_{BC} = \frac{84.98}{EI}$$

$$M_{AB} = 25.4 \text{ k} \cdot \text{ft} \quad \text{Ans}$$

$$M_{BA} = 64.3 \text{ k} \cdot \text{ft} \quad \text{Ans}$$

$$M_{BC} = -64.3 \text{ k} \cdot \text{ft} \quad \text{Ans}$$

$$M_{CB} = 99.8 \text{ k} \cdot \text{ft} \quad \text{Ans}$$

$$M_{CD} = -99.8 \text{ k} \cdot \text{ft} \quad \text{Ans}$$